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LOUISIANA-PACIFIC CORPORATION / WOOD-E DESIGN 2016.1 09/18/18 09:48:36

WARNING

*** THIS DESIGN IS VALID FOR THE PROJECT NAMED BELOW (JOB ID) ONLY

*** WOOD-E DESIGN 2016.1 EXPIRES ON 3/31/2018. LP WILL MAKE AVAILABLE TO ALL REGISTERED USERS AN UPDATED VERSION OF THE WOOD-E DESIGN SOFTWARE IN THE CONTINUING EFFORT TO MAINTAIN COMPLIANCE WITH CHANGING BUILDING CODES, INDUSTRY PRACTICES, CODE EVALUATION REPORTS AND/OR METHODS OF ANALYSIS.

COMPANY:

JOB ID: Krutsinger BE2 STATE: FL CODE: FBC '10

PRODUCT: 2-PLY 1-3/4" X 11-1/4" LP LSL 1.55E

DESIGN CRITERIA FOR ROOF BEAM (UNFACTORED LOADS)

LIVE	DEAD	SPAN (L)	SPAN (R)			ALLOWABLE	DEFLECTION
(PSF)	(PSF)	CARRIED	CARRIED	SLOPE	LOADING	LIVE	TOTAL
20	20	0.000'	0.000'	0.00	TOP	L/240	L/180
SPAN C	ARRIED	IS NOT CON	TINUOUS.				

ALLOWABLE / WORL	DEFLECTION				
	REACTION	MOMENT	SHEAR	LIVE LOAD	TOTAL LOAD
ACTUAL	3973	17274	3973	0.422	0.860
ALLOWABLE	4594	18291	13453	0.677	0.903
STRESS INDICES	0.86	0.94	0.30	L/385	L/189
LOAD CASE	1	1	1	1	1

**** THE REACTION, MOMENT AND SHEAR DATA ABOVE ARE BASED ON THE MAXIMUM STRESS INDICES AND MAY NOT REFLECT THE ABSOLUTE MAXIMUM ACTUALS.

**** ALLOWABLE DEFLECTIONS ARE BASED ON THE DESIGN SPAN LENGTH (L) OR TWICE THE LENGTH FOR CANTILEVERS (2L).

CONNECTION

- *** DESIGN ASSUMES ALL "TOP" LOADS ARE APPLIED TO TOP EDGE OF BEAM, SUCH THAT LOAD IS DISTRIBUTED EQUALLY TO EACH PLY.
- ***ATTACH THE TWO PLIES WITH 2 ROWS OF 16d (3-1/2") NAILS AT 12" OC. STAGGER ROWS. NAILS CAN BE DRIVEN FROM ONE FACE OR HALF FROM EACH FACE. NAILS MAY BE COMMON OR BOX NAILS WITH A MINIMUM SHANK DIAMETER OF 0.131" 16d SINKERS (3-1/4") MAY BE USED, BUT HALF MUST BE DRIVEN FROM EACH FACE.
- *** CONCENTRATED LOADS MUST BE EQUALLY DISTRIBUTED TO ALL PLIES. ADDITIONAL FASTENERS MAY BE REQUIRED.

NOTES

- *** COMPRESSION EDGE BRACING REQUIRED AT 24" O.C. OR LESS.
- *** THIS LSL BEAM HAS BEEN DESIGNED TO SUPPORT A 300 LBS CONCENTRATED LOAD ACTING OVER 2.5 X 2.5 FT (6.25 SQ.FT)

STRUCTURAL GEOMETRY

SPAN 1

13.670'

TOTAL SPAN: 13.67 FT

LOAD PATTERNS

CASE	SPAN	SHAPE	TYPE	SOURCE	LOADING	W1	W2	X1 (FT) X2	(FT)
+ALL	1	UNIF	WEIGHT	BEAM		12.6 PLF	i	0.000	13.	670

Page 2 Krutsinger BE2.dsn TOP 1680.0 LBS 7.000 ALL 1 CONC DEAD ROOF TOP ALL 1 CONC DEAD ROOF ALL 1 CONC DEAD ROOF ALL 1 CONC DEAD ROOF 468.0 LBS 1.500 TOP 468.0 LBS TOP 468.0 LBS 5.500 CONC DEAD ROOF TOP 174.0 LBS ALL 1 9.500 ALL 1 CONC DEAD ROOF TOP 140.0 LBS 11.500 ALL 1 CONC DEAD ROOF TOP 80.0 LBS 13.000 1680.0 LBS 468 0 LBS CONC LIVE ROOF TOP CONC LIVE ROOF TOP CONC LIVE ROOF TOP TOP TOP 1 1 7.000 468.0 LBS 468.0 LBS 1 1 3.500 1 1 CONC LIVE ROOF TOP 468.0 LBS 1.500 174.0 LBS 1 1 CONC LIVE ROOF TOP 9.500 1 CONC LIVE ROOF 1 CONC LIVE ROOF 140.0 LBS TOP 1 11.500 TOP 80.0 LBS 1 13.000 0.0 LBS 2 1 CONC LIVE ROOF TOP 7.000 2 1 CONC LIVE ROOF TOP 0.0 LBS 1.500 2 1 CONC LIVE ROOF TOP 0.0 LBS 3.500 CONC LIVE ROOF CONC LIVE ROOF 2 TOP 0.0 LBS 5.500 1 0.0 LBS 2 1 TOP 9.500 CONC LIVE ROOF 2 TOP 0.0 LBS 11.500 1 2 CONC LIVE ROOF TOP 0.0 LBS 13.000 3 1 UNIF LIVE ROOF TOP 48.0 PLF 0.125 2.625 4 UNIF LIVE ROOF TOP 48.0 PLF 11.045 13.545 1 UNIF LIVE 5 1 ROOF TOP 48.0 PLF 5.585 8.085 + INDICATES LOAD IS BASED ON SPAN CARRIED AND INPUT LIVE OR DEAD LOAD PSF.

SECTION FORCES

CASE	MOMENT (LB-FT)	SHEAR (LBS)	LDF	LOAD COMBINATIONS
1 2 3 4 5	17274 8781 8860 8860 9150	3973 2029 2137 2041 2089	0.90 1.25 1.25	D+Lr D D+Lr D+Lr D+Lr

UNFACTORED SUPPORT REACTIONS (LBS) USE THESE VALUES WHEN DESIGNING CONNECTORS

REACTIONS FOR TOTAL LOADS

CASE	BRG#1	BRG#2
1	3973	3155
2	2029	1620
3	2137	1633
4	2042	1728
5	2089	1680

REACTIONS FOR DEAD LOAD

CASE	BRG#1	BRG#2
1	2029	1620
2	2029	1620
3	2029	1620
4	2029	1620
5	2029	1620

REACTIONS FOR LIVE LOAD

CASE	BRG#1	BRG#2
1	1943	1535
2	0	0
3	108	12
4	12	108
5	60	60

MINIMUM BEARING SIZES (IN)

BRG# 1	BRG# 2
1.500	1.500

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SPAN	TYPE	W1	(LBS)	X1	(FT)	MIN	BRG	(IN)
1	DEAD	168	0.0	7.0	000	3.00	0.0	
1	DEAD	468	.0	1.5	00	3.00	0.0	
1	DEAD	468	.0	3.5	00	3.00	0.0	
1	DEAD	468	.0	5.5	00	3.00	0.0	
1	DEAD	174	. 0	9.5	00	3.00	0.0	
1	DEAD	140	.0	11.	500	3.00	0.0	
1	DEAD	80.	0	13.	000	3.00	0.0	
1	LIVE	168	0.0	7.0	000	3.00	0.0	
1	LIVE	468	.0	5.5	00	3.00	0.0	
1	LIVE	468	.0	3.5	00	3.00	0.0	
1	LIVE	468	.0	1.5	00	3.00	0.0	
1	LIVE	174	. 0	9.5	00	3.00	0.0	
1	LIVE	140	.0	11.	500	3.00	0.0	
1	LIVE	80.	0	13.	000	3.00	0.0	

CASE	SPAN	LIVE LOAD			TOTAL LOA	AD DEFLE ALLOW.		DEAD LOA	AD DEFLECTION LONG-TERM
1	1	0.422	0.677	L/385	0.860	0.903	L/189		
2	1	0.000	0.677		0.438	0.903	L/371	0.438	0.657
3	1	0.005	0.677	L/31335	0.443	0.903	L/367		
4	1	0.005	0.677	L/31335	0.443	0.903	L/367		
5	1	0.018	0.677	L/9127	0.456	0.903	L/356		

**** FOR DEAD LOAD DEFLECTION DATA SEE LOAD CASE 2 ****
**** TOTAL LOAD DEFLECTION SHOWN IS INSTANTANEOUS. ****

**** ALLOWABLE DEFLECTIONS ARE BASED ON THE DESIGN SPAN LENGTH (L) OR TWICE THE LENGTH FOR CANTILEVERS (2L).

STRESS INDICES	CASE	MSI	VSI
	1	0.94	0.30
	2	0.67	0.21
	3	0.48	0.16
	4	0.48	0.15
	5	0.50	0.16

SLENDERNESS RATIO = 3.21 LIMIT = 10.00

VERIFY YOUR INPUT TO AVOID DESIGN AND FABRICATION MISTAKES. YOU ARE SOLELY RESPONSIBLE FOR ERRORS RESULTING FROM INCORRECT INPUT. THIS PROGRAM IS A DESIGN TOOL AND SHOULD BE USED WITH EXTREME CARE THAT INPUT UNIFORM AND CONCENTRATED LOADS ARE ACCURATE IN MAGNITUDE AND LOCATION. IF YOU HAVE ANY QUESTIONS OR UNCERTAINTIES, PLEASE CONTACT LP.

THIS COMPONENT DESIGN IS SPECIFICALLY FOR LP ENGINEERED WOOD PRODUCTS. USE OF THIS PROGRAM TO DESIGN ANYTHING OTHER THAN LP LVL, LP LSL, OR LPI-JOISTS IS STRICTLY PROHIBITED. LP IS A TRADEMARK OF LOUISIANA-PACIFIC CORPORATION.

Krutsinger BE2 FL

2016.1 Allowable Stress Design NOTE: 1. THIS COMPONENT IS DESIGNED TO SUPPORT ONLY THE VERTICAL LOADS SHOWN VERIFICATION OF LOADING, DEFLECTION LIMITATIONS, FRAMING METHODS, WIND AND SEISMIC BRACING, AND OTHER LATERAL BRACING THAT IS ALWAYS REQUIRED IS THE RESPONSIBILITY OF THE PROJECT ENGINEER OR ARCHITECT. 2. PROVIDE RESTRAINT AT SUPPORTS TO ENSURE LATERAL STABILITY 3. DO NOT CUT, NOTCH OR DRLL LP LSL. 4. SHIM ALL BEARINGS FOR FULL CONTACT. 5. VERIFY DIMENSIONS BEFORE CUTTING LP LSL TO SIZE 6. THIS LP LSL IS TO BE USED AS A ROOF BEAM ONLY. MAKE PROVISION FOR ADEQUATE DRANAGE. '. COMPRESSION EDGE BRACING REQUIRED AT 24" O.C. OR LESS.

DESIGN ASSUMES ALL "TOP" LOADS ARE APPLIED TO TOP EDGE OF LP LSL, SUCH THAT LOAD IS DISTRIBUTED FOUALLY TO EACH PLY ATTACH THE TWO PLIES WITH 2 ROWS OF 16d (3-1/2") NAILS AT 12" OC. STAGGER ROWS. NAILS CAN BE DRIVEN FROM ONE FACE OR HALF FROM EACH FACE. NAILS MAY BE COMMON OR BOX NAILS WITH A MINIMUM SHANK DIAMETER OF 0.131". 16d SINKERS (3-1/4") MAY BE USED, BUT HALF MUST BE DRIVEN FROM EACH FACE.

CONCENTRATED LOADS MUST BE EQUALLY DISTRIBUTED TO ALL PLIES. ADDITIONAL FASTENERS MAY BE REQUIRED.

LOAD TABLE NOTE: LOADS SHOWN ARE FOR INPUT LOAD CASE (1). OTHER LOAD CASES

FOR PATTERN LIVE LOADING ARE CHECKED AS REQUIRED. (DIMENSIONS MEASURED FROM LEFT END OF SPAN OR CANTILEVER.)

DISTRIBUTION SOURCE TYPE TOP/SIDE LOAD FROM

T.TVE

DEAD

DEAD

LIVE

DISTRIBUTION	SOURCE	TIPE I	OF/SIDE	LOAD		FROM	10	LOAD	LDE	THOEL
						FT-IN-SX	FT-IN-SX			
UNIFORM	BEAM	WEIGHT		13	PLF	00-00-00	13-08-01		0.90	
CONCENTRATED	ROOF	DEAD	TOP	468	LBS	01-06-00	MINBRG=3.	00"	0.90	
CONCENTRATED	ROOF	LIVE	TOP	468	LBS	01-06-00	MINBRG=3.	00"	1.25	
CONCENTRATED	ROOF	LIVE	TOP	468	LBS	03-06-001	MINBRG=3.	00"	1.25	
CONCENTRATED	ROOF	DEAD	TOP	468	LBS	03-06-00	MINBRG=3.	00"	0.90	
CONCENTRATED	ROOF	LIVE	TOP	468	LBS	05-06-00	MINBRG=3.	00"	1.25	
CONCENTRATED	ROOF	DEAD	TOP	468	LBS	05-06-001	MINBRG=3.	00"	0.90	
CONCENTRATED	ROOF	LIVE	TOP	1680	LBS	07-00-00M	MINBRG=3.	00"	1.25	Girder
CONCENTRATED	ROOF	DEAD	TOP :	1680	LBS	07-00-00M	MINBRG=3.	00"	0.90	Girder
CONCENTRATED	ROOF	LIVE	TOP	174	LBS	09-06-00	MINBRG=3.	00"	1.25	
CONCENTRATED	ROOF	DEAD	TOP	174	LBS	09-06-00	MINBRG=3.	00"	0.90	

140 T.BS 11-06-00MTNBRG=3.00"

140 LBS 11-06-00MINBRG=3.00"

80 LBS 13-00-00MINBRG=3.00"

80 LBS 13-00-00MINBRG=3.00"

WARNING NOTES:

CONCENTRATED ROOF

CONCENTRATED ROOF

CONCENTRATED ROOF

CONCENTRATED ROOF

THIS COMPONENT DESIGN IS SPECIFICALLY FOR L-P ENGINEERED WOOD PRODUCTS. USE OF THIS DESIGN FOR ANYTHING OTHER THAN LP LVL OR LP LSL OR LP I-JOISTS IS STRICTLY PROHIBITED. ANY MODIFICATION OF THIS DOCUMENT REQUIRES REVIEW BY A DESIGN PROFESSIONAL.

TOP

TOP

TOP

TOP

PROVIDE RESTRAINT AT CONCENTRATED LOAD TO ENSURE LATERAL STABILITY.

MINIMUM BEARING SIZES ARE SUFFICIENT TO PREVENT CRUSHING OF THE LP LSL BEAM AS DESIGNED. IT IS THE RESPONSIBILITY OF THE PROJECT ENGINEER, ARCHITECT OR DESIGNER TO VERIFY THAT THE SUPPORT STRUCTURE FOR THIS BEAM IS CAPABLE OF SUPPORTING THE REACTIONS.

ANCHOR LP LSL ROOF BEAM SECURELY TO BEARINGS OR HANGERS.

THIS COMPONENT MEETS CODE ALLOWED DEFLECTION CRITERIA; CALCULATED DEFLECTION EXCEEDS 3/4" AND SHOULD BE REVIEWED BY PROJECT DESIGNER FOR ADEQUACY.

LP LSL ROOF BEAMS ARE MANUFACTURED WITHOUT CAMBER, THEREFORE, IN ADDITION TO COMPLYING WITH THE DEFLECTION LIMITS OF LOCAL BUILDING CODES, OTHER DEFLECTION CONSIDERATIONS SHOULD BE EVALUATED BY THE PROJECT ENGINEER OR ARCHITECT SUCH AS PONDING, CRACKING AND AESTHETICS. (POSITIVE DRAINAGE IS ESSENTIAL)

THIS LSL BEAM HAS BEEN DESIGNED TO SUPPORT A 300 LBS CONCENTRATED LOAD ACTING OVER 2.5 X 2.5 FT (6.25 SQ FT)

12 0 CROSS SECTION - 13- 8- 1- *** THIS DRAWING IS NOT TO SCALE ***

User Notes (User is responsible for the accuracy of these notes)

2 PLIES 1.750 X 11.250 LP LSL 1.55E

TOGETHER (REFER TO NOTES).

LDF LABEL

1.25 Rafter 1

0.90 Rafter 1

0.90

1.25

T.OAD

DESIGN CONSISTS OF 2 - PLIES FASTENED

SUPPORT REACTIONS (LBS): MAXIMUM BEARING NUMBER DOWN 3973 3155 TIPT.TET

MIN BEARING SIZES (IN-SX)

Handling & Erection

MAXIMUM DEFLECTIONS CALCULATED ALLOWABLE LIVE LOAD 0.42"(L/385) 0.68" *DEAD LOAD 0.66" TOTAL LOAD 0.86" (L/189) 0.90"

to the component until after all the framing and fastening are completed. At no time shall loads greater than design loads be applied to the component. Design Criteria The design and material specified are in substantial conformity with the latest revisions of NDS.* Dead load deflection includes adjustment factor for creep. Total

Temporary and permanent bracing for holding component plumb and for resisting Donotout, notch, drill or alter LP SolidStartLSL, LVL and I-Joists except as shown lateral forces shall be designed and installed by others. No loads are to be applied in published material from LP. Any use of LP SolidStart LSL, LVL and LJoists contrary to the limits set forth hereon, negates any express warranty of the product and LP disclaims all implied warranties including the implied warranties of merchantability and fitness for a particular use.

LP® SolidStart® LSL LVL and Lloist Specifications

A COPY OF THIS DRAWING IS TO BE GIVEN TO THE INSTALLING CONTRACTOR.

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Software Provided Bv: LP Engineered Wood Products 414 Union Street, Suite 2000 Nashville, TN 37219 Phone 800 515 7570

Fax 866,753,4369

DWG

SHEET #

09/18/18

FBC '10

See also sheet.1 of

MSI: 0.94

VSI: 0.30

RST: 0.86

L / 240

L / 180

PSF

20

20 PSF

40 PSF

DESIGN CRITERIA :

DEFLECTION CRITERIA :

LIVE LOAD DEFL:

TOTAL LOAD DEFL:

CODE COMPLIANCES :

ROOF LEFT SPAN CARR. : 0.00 FT

ROOF RIGHT SPAN CARR : 0.00 FT

REPORT #

PR-L280

ESR-2403

RR-25783

13319-R

FL15228

LIVE LOAD

DEAD LOAD

TOTAL LOAD

APA

ICC-ES

Florida

T.ADBS

CCMC

Krutsinger BE2					FL			
1.6	OAD TABLE							
		Thou so						
DISTRIBUTION SOURCE TYPE TOP/S	SIDE LOAD F	FROM TO FT-IN-SX FT-IN-SX	LOAD LDF	LABEL				
						Software Provided Bv:	Опочо	a 110
						Software Provided By: LP Engineered Wood Products	09/18/18 FB0	C .IO
						414 Union Street, Suite 2000 Nashville TN 37219		
						414 Union Street, Suite 2000 Nashville, TN 37219 Phone 800.515.7570 Fax 866.753.4369		
						Fax 866.753.4369		
						DWG #		
								_
						SHEET #		_